

## The New Method in Calculating Columns and Beams Dimensions That Meets Requirements of The Strong Column - Weak Beam and Non - Soft Story

Livian Teddy<sup>1\*</sup>, Gagoek Hardiman<sup>2</sup>, N. Nuroji<sup>3</sup>, Sri Tudjono<sup>3</sup>

<sup>1</sup>Department of Architecture, Universitas Sriwijaya, Palembang, Indonesia
 <sup>2</sup>Department of Architecture, Universitas Diponegoro, Semarang, Indonesia
 <sup>3</sup>Department of Civil Engineering, Universitas Diponegoro, Semarang, Indonesia

\*Corresponding e-mail: livianteddy@gmail.com

Article info:

Received: 15-3-2019, Revised: 22-3-2019, Accepted: 26-3-2019

Abstract. Situated at an earthquake prone area, buildings planning in Indonesia must implement earthquake resistant building principles. One of these principles is determining dimensions of columns and beams in the process of architectural designing. This act eventually affects the behaviour of the strong column - weak beam and the probability of bending failure due to soft story. At present time, there are no simple rules architects can use in calculating the dimensions of beams and columns that meet the criteria for strong column - weak beam and non - soft story. This paper is an effort to provide an input to the architects in designing the dimensions of the columns and beams. This research is a review result of three theories namely: 1). The theory of columns and beams preliminary design, 2). The theory of the strong column - weak beam concept, and 3). The theory of soft story and column slenderness. Those theories were then synthesized into a spreadsheet. To meet the criteria for strong column - weak beam and non - soft story, the following procedures must be done : 1). Determine the columns' dimensions according to 0.15% of the columns' cumulative tributary area, 2). Determine the beams' dimensions according to 1/12 of the beams' span and the beams' plastic modulus, 3). Determine the columns' dimensions and the columns' plastic modulus, 4). Determine the columns' height based on the column slenderness criteria, and 5). Compare the columns plastic modulus and the beams plastic modulus and check whether they meet the criteria "the columns'plastic modulus ≥ 1.2 \* the beams' plastic modulus".

Keywords: strong column - weak beam, soft story, column slenderness

#### 1. Introduction

Indonesia is an earthquake prone area, so buildings planning in this country must be in accordance with earthquake-resistant building principles. The planning process for earthquake resistant buildings should start from its architectural design process, and the determination the columns and beams dimensions should be the first step because this important step influences the behaviour of the strong column - weak beam and the probability of bending failure as a result of the soft story. Strong column - weak beam is a concept that describes a condition where plastic hinge occurs only in beams and not in columns when a strong earthquake happens, hence avoiding sudden collapse (BSN, 2013). On the other hand, soft story is a form of geometric irregularity of a building where one story has lower lateral stiffness than the other stories (Arnold, 2001).

At present there are no simple rules architects can apply in calculating the dimensions of the beams and columns to meet the criteria for strong column - weak beam and to avoid the soft story. This paper strives to provide input to architects in creating preliminary design of beams and columns dimensions that satisfies the principles of making earthquake resistant building.

The concept of a strong column - weak beam is that the calculation result of nominal moments of the upper and lower columns must be 1.2 times bigger than the sum of nominal

moments of the left and right supporting beams (see figure 1.1). The purpose of this concept is to prevent the occurrence of the plastic hinge in the columns that serve as the main component of lateral load supports. Therefore, melting should only occur in the beams so that when there is very large inelastic deformation, no collapse happens.



Figure 1.1. The concept of strong column - weak beam (source: BSN, 2013)

To simplify its application in the structure preliminary design, the following formula is used (Bisch et al., 2012) :

Wp of columns  $\geq$  1.2\*Wp of beams (1)

In which, Wp = plastic modulus (cm3 or m3) Wp = 1/4xbxh2 b = the beams'/columns' width h = the beams/columns' height

Soft story occurs if there is a story having lateral stiffness 70% (soft story) or 60% (excessive soft story) less than the lateral stiffness of the story above it (FEMA, 2007). Soft story is usually related to the height of the floor and the slenderness of the column. Soft story may occur if the height of a floor is higher than the height of other floors; in other words this floor is less stiff than the other floors (Arnold, 2001). To overcome this problem, the slenderness of the column must be limited (see table 1.1).

Tabel 1.1. The criteria for slenderness of columns (Sources : compiled from Seki 2015; Okada et al.

	•	
200	5)	١.

Types of Lateral Elements	Requirem	ents
Columns	The columns' net height/dimensions; h0/D	Definition h0/D
a). Slender	6≤h0/D	
b). Normal	2 <h0 d<6<="" td=""><td>h0</td></h0>	h0
c). Short	h0/D≤2	

In which, h0 = the columns'net height

 $h = h0 - h_b$ 

D = the reviewed columns' width

For columns, the minimum dimensions are based on the consideration of their shear strength (Ersoy, 2013) :

$$A_c \ge 0.0015 \sum A_o \tag{2}$$

$$b_c = h_c = \sqrt{A_c} \tag{3}$$

In which,

 $\begin{array}{l} A_{\rm c} = \mbox{the columns' cross-sectional area} \\ A_{\rm o} = \mbox{the columns' tributary area} \ (\mbox{see figure 1.2}) \\ \sum A_{\rm o} = \mbox{the columns' cumulative tributary area} \\ A_{\rm c} \ge 0.09 \ \mbox{m2 (min 30x30 cm)} \\ b_{\rm c} = \mbox{the columns' width} \\ h_{\rm c} = \mbox{the columns' height} \end{array}$ 

In general, the preliminary dimensions of one-way and two-way beams can be calculated with the following formula:

$$h_{\rm b} = 1/10 - 1/14^* L \tag{4}$$

$$b_b \ge \frac{1}{2} h_b \tag{5}$$

In which,  $h_b$  = the beams' height  $b_b$  = the beams' width (min  $b_b \ge 25$  cm)

L = the beams' span



Figure 1.2. The columns' tributary area of A1, A2, A3 & A4

#### 2. Methods

This research is the result of a literature review. The theories reviewed were : 1). The theory of columns and beams preliminary design, 2). The theory of the strong column - weak beam concept, and 3). The theory of soft story and column slenderness. These theories were then synthesized into a spreadsheet to be a new proposal for calculating dimensionsof columns and beams able to fulfil the requirements for the strong column - weak beam and non - soft story.

#### 3. Result & Discussion

#### 3.1. Calculate The Dimensions of The Beams and Columns

Floors	$A_{o}(m^{2})$	∑A <sub>o</sub> (m²)	$A_{c}(m^{2})$
1 <sup>st</sup>	35	350	0.525
2 <sup>nd</sup>	35	315	0.473
3 <sup>rd</sup>	35	280	0.420
4 <sup>th</sup>	35	245	0.368
5 <sup>th</sup>	35	210	0.315
6 <sup>th</sup>	35	175	0.263
7 <sup>th</sup>	35	140	0.210
8 <sup>th</sup>	35	105	0.158
9 <sup>th</sup>	35	70	0.105
10 <sup>th</sup>	35	35	0.053

 Table 3.1. Determining of the columns cross-sectional area

It is assumed that a building consists of 10 floors with a 5x7 meters structure module, and we need to calculate the dimensions of the beams and columns that meet the criteria for the strong column - weak beam and non - soft story. To solve this problem, we need to do following steps:

- Step 1, see table 3.1 and determine the columns' tributary area (A<sub>o</sub>) and the columns' cumulative tributary area ( $\sum A_o$ ). Next, obtain the columns' cross-sectional area (A<sub>c</sub>) based on formula (2). For example, for the 1<sup>st</sup> floor (A<sub>o</sub>) = 5 \* 7 = 35 m<sup>2</sup>, it is assumed that the typical column tributary area per floor ( $\sum A_o$ ) = 35 \* 10 = 350 m<sup>2</sup>, next the columns' cross-sectional area of the 1<sup>st</sup> floor (A<sub>c</sub>) = 0.15% \*  $\sum Ao = 0.15\%$  \* 350 = 0.525 m<sup>2</sup>.
- Step 2, see table 3.2. For the beams'span (Lx), determine the beams' height (h<sub>b</sub>) based on formula (4) and the beams' width (b<sub>b</sub>) based on formula (5). Then obtain the beams' plastic modulus (Wp<sub>b</sub>) based on formula (1). For example, the 1<sup>st</sup> floor (Lx) = 7 m. Calculate the height and the width of the beams, h<sub>b</sub> = 1/12 \* Lx = 1/12 \* 7 = 0.58 ≈ 0.6 m and b<sub>b</sub> = 1/2 \* h<sub>b</sub> = 1/2 \* 0.6 = 0.3 m. Calculate the plastic modulus of the beams (Wp<sub>b</sub>) =  $1/4 * b_b * h_b^2 = 1/4 * 0.3 * 0.6^2 = 0.0270 \text{ m}^3$ .
- Step 3, see table 3.2. For the columns' cross-sectional area (A<sub>c</sub>) obtained in step 1, determine the columns' dimension (b<sub>c</sub>) based on the formula (3) and determine the columns'plastic modulus(Wp<sub>c</sub>) based on the formula (1). For example, for the columns' cross-sectional area of the 1st floor (A<sub>c</sub>) = 0.525 m<sup>2</sup> and their dimensions (b<sub>c</sub>) =  $\sqrt{A_c} = \sqrt{0.525} = 0.72 \approx 0.75$  m. Calculate the their plastic modulus (Wp<sub>c</sub>) = 1/4 \* b<sub>c</sub> \* h<sub>c</sub><sup>2</sup> = 1/4 \* 0.75 \* 0.75<sup>2</sup> = 0.1055 m<sup>3</sup>.

- Stage 4, see table 3.2. Limit the columns' maximum height to 5 m and the columns' minimum height to 3 m. Determine the maximum and minimum height of the columns based on the formula (b) table 1. For example, the maximum columns' height for  $1^{st}$  ( $h_{max}$ ) = (6 \*  $b_c$ ) +  $h_b$  = (6 \* 0.75) + 0.6 = 5.1 ≈ 5 m while the minimum columns' height ( $h_{min}$ ) = (2 \*  $b_c$ ) +  $h_b$  = (2 \* 0.75) + 0.6 = 2.1 ≈ 3 m.
- Stage 5, see table 3.2. Compare the columns' plastic modulus ( $W_{pc}$ ) and the beams' plastic modulus ( $Wp_b$ ) based on the formula (1). For example, for the 1<sup>st</sup> floor,  $Wp_c = 0.1055 \text{ m}^3$  and  $Wp_b = 0.0270 \text{ m}^3$ , in which 0.1055 > 1.2 \* 0.0270 = 0.0324 hence fulfilling  $Wp_c$ 's requirements > 1.2xWp<sub>b</sub>.

		В	eams			Colum	S	Colu hei	mns' ght	Columns> beams
Floors	Lx (m)	h <sub>ь</sub> (m)	b <sub>ь</sub> (m)	Wp <sub>b</sub> (m³)	A <sub>c</sub> (m²)	b <sub>c</sub> (m)	Wp <sub>c</sub> (m³)	h max (m)	h min (m)	Wp <sub>c</sub> >1.2xWp <sub>b</sub>
1 <sup>st</sup>	7	0.60	0.30	0.0270	0.525	0.75	0.1055	5.00	3.00	Fulfil
2 <sup>nd</sup>	7	0.60	0.30	0.0270	0.473	0.70	0.0858	4.80	3.00	Fulfil
3 <sup>rd</sup>	7	0.60	0.30	0.0270	0.420	0.65	0.0687	4.50	3.00	Fulfil
4 <sup>th</sup>	7	0.60	0.30	0.0270	0.368	0.60	0.0540	4.20	3.00	Fulfil
5 <sup>th</sup>	7	0.60	0.30	0.0270	0.315	0.55	0.0416	3.90	3.00	Fulfil
6 <sup>th</sup>	7	0.60	0.30	0.0270	0.263	0.50	0.0313	3.60	3.00	Does not fulfil
7 <sup>th</sup>	7	0.60	0.30	0.0270	0.210	0.45	0.0228	3.30	3.00	Does not fulfil
8 <sup>th</sup>	7	0.60	0.30	0.0270	0.158	0.40	0.0160	3.00	3.00	Does not fulfil
9 <sup>th</sup>	7	0.60	0.30	0.0270	0.105	0.35	0.0107	2.70	3.00	Does not fulfil
10 <sup>th</sup>	7	0.60	0.30	0.0270	0.053	0.25	0.0039	2.10	3.00	Does not fulfil

Table 3.2. Determining of the beams and columns dimensions on the x-axis

From stage 1 to stage 5, the overall results can be seen in table 3.2. From the 1<sup>st</sup> floor to the 10<sup>th</sup> floor, only the 1<sup>st</sup> floor until the 5<sup>th</sup> floor that meet the criteria for the strong column - weak beam. With the same span (Lx) = 7 m, the dimensions of the beams from the 1<sup>st</sup> to the 10<sup>th</sup> floors are same, namely  $b_b = 0.3$  m and  $h_b = 0.6$  m. The columns' dimensions ( $b_c$ ) from1<sup>st</sup> floor until the 5<sup>th</sup> floor may vary according to the weight they support. The columns' dimensions can be determined based on the criteria for the strong column - weak beam and the columns' height. If it is based on the criteria of the strong column - weak beam and it is assumed that the height of the column for each floor (h) = 4 m, then only the 1<sup>st</sup> floor to the 4<sup>th</sup> floor to the 4<sup>th</sup> floor = 0.75x0.75 m and for the 5<sup>th</sup> floor until the 10<sup>th</sup> floor = 0.60x0.60 m.

Table 3.3. Determining of the beams an	d columns dimensions on the y-a	axis
--	---------------------------------	------

		В	eams			Columr	IS	Colu hei	mns' ght	Columns> beams
Floors	Lx (m)	h₀ (m)	b₅ (m)	Wp₅ (m³)	$A_c$ $(m^2)$	b <sub>c</sub> (m)	Wp <sub>c</sub> (m³)	h max (m)	h min (m)	Wp <sub>c</sub> >1.2*Wp <sub>b</sub>
1 <sup>st</sup>	5	0.45	0.25	0.0127	0.525	0.75	0.1055	4.95	3.00	Fulfil
2 <sup>nd</sup>	5	0.45	0.25	0.0127	0.473	0.70	0.0858	4.65	3.00	Fulfil

Journal of Architectural Design and Urbanism Vol 1, No 2, March 2019

Table 3.3. Con	tinue									
3 <sup>rd</sup>	5	0.45	0.25	0.0127	0.420	0.65	0.0687	4.35	3.00	Fulfil
4 <sup>th</sup>	5	0.45	0.25	0.0127	0.368	0.60	0.0540	4.05	3.00	Fulfil
5 <sup>th</sup>	5	0.45	0.25	0.0127	0.315	0.55	0.0416	3.75	3.00	Fulfil
6 <sup>th</sup>	5	0.45	0.25	0.0127	0.263	0.50	0.0313	3.45	3.00	Fulfil
7 <sup>th</sup>	5	0.45	0.25	0.0127	0.210	0.45	0.0228	3.15	3.00	Fulfil
8 <sup>th</sup>	5	0.45	0.25	0.0127	0.158	0.40	0.0160	2.85	3.00	Does not fulfil
9 <sup>th</sup>	5	0.45	0.25	0.0127	0.105	0.35	0.0107	2.55	3.00	Does not fulfil
10 <sup>th</sup>	5	0.45	0.25	0.0127	0.053	0.25	0.0039	1.95	3.00	Does not fulfil

On the -y axis (table 3.3), only the 1<sup>st</sup> floor until the 7<sup>th</sup> floor meet the criteria for strong column - weak beam. With the same span (Ly) = 5 m, the dimensions of the beams of all floors are the same, namely  $b_b = 0.25$  m and  $h_b = 0.45$  m. The column dimensions ( $b_c$ ) on the x-axis and y-axis that meet the criteria for the strong column - weak beam and column height vary, so the most critical part is chosen, namely the x-axis with the columns' dimensions of the 1<sup>st</sup> floor to the 4<sup>th</sup> floor = 0.75x0.75 m and of the 5<sup>th</sup> floor to the 10<sup>th</sup> floor = 0.60x0.60 m.

# 3.2. The Soft Story is One of The Most Dangerous Types of Building Irregularity

The aforementioned procedure of calculating columns and beams dimensions that fulfils the strong column - weak beam and non-soft requirements are expected to reduce the potential soft story because the soft story is one of the most dangerous types of building irregularity (Mezzi, 2006). Boen researches in Padang, Bengkulu and Yogyakarta (Boen, 2006, 2007a, 2007b) show that the most frequent and deadly structural failure due to large earthquakes is the soft story (see figure 3.1).



Figure 3.1. The Padang earthquake on September 30 & October 1, 2007: a). The Office of DPU Padang experienced soft story on the 1<sup>st</sup> floor but it was still survived, b). The 3 - story-shophouse experienced severe soft story at the 1<sup>st</sup> floor. This floor collapsed, and the 2<sup>nd</sup> & 3<sup>th</sup> floors fell on it, c). & d). Two commercial buildings experiencing severe soft story both at the 1st floor. The 1<sup>st</sup> floors collapsed, and the 2<sup>nd</sup> and 3<sup>rd</sup> floors fell on them (source:

http://www.perencanaanstruktur.com/2010/10/bentuk-keruntuhan-bangunan-saat-gempa.html).

### 4. Conclusion

To make columns and beams dimensions meet the criteria for the strong column - weak beam and non - soft story, the following steps must be conducted:

- Determine the columns' dimensions based on 0.15% of the columns' cumulative tributary area.
- Determine beams' dimensions based on 1/12 of their span and beams' plastic modulus.
- Determine columns' dimensions and columns' plastic modulus.
- Determine columns' height based on the columns' slenderness criteria.
- Compare the plastic modulus of both columns and beams and find out whether the result satisfies the criteria "columns'plastic modulus ≥ 1.2 \* the beams' plastic modulus".

With this new proposed way in calculating dimensions of the columns and beams, we can expect the number of sudden collapse due to the occurrence of plastic hinge and soft stories in the columns can be reduced.

#### Glossary :

•	Column / beam plastic modulus	:	used for materials where elastic yielding is acceptable and plastic behavior is assumed to be an acceptable limit.
•	Plastic hinge	:	a yielding zone in an structural elements which generally develops at the point of maximum bending moment, support, etc.
•	Slender column	:	if its cross-sectional dimensions are small compared with its length.
•	Soft story	:	if there is a story having lateral stiffness 70% (soft story) or 60% (excessive soft story) less than the lateral stiffness of the story above it.
•	Strong column - weak beam	:	a concept that describes a condition where plastic hinge occurs only in beams and not in columns when a strong earthquake happens, hence avoiding sudden collapse.

#### 5. References

- Alwashali, H., & Maeda, M. (2012). Study of Seismic Evaluation Methods Of RC Buildings With Masonry Infill Walls; A Case Study of Building in Jordan. In Japan Association for Earthquake Engineering (JAEE) (Ed.), *International Symposium on Earthquake Engineering, JAEE, Vol.1* (pp. 497–506). Tokyo: JAEE.
- Arnold, C. (2001). Architectural Considerations. In F. Naeim (Ed.), *The Seismic Design Handbook*. Boston, MA: Springer US. https://doi.org/10.1007/978-1-4615-1693-4
- Bisch, P., Carvalho, E., Degee, H., Fajfar, P., Fardis, M., Franchin, P., ... Tsionis, G. (2012). *Eurocode 8: Seismic Design of Buildings Worked examples*. Luxembourg: https://doi.org/10.2788/91658

Boen, T. (2006). The Yogya Earthquake 27 May 2006, Structural Damage Report.

- Boen, T. (2007a). Bengkulu & West Sumatra Earthquakes, September 12, 2007, Structural Damage Report.
- Boen, T. (2007b). West Sumatra Earthquake, 6 March 2007, Structural Damage Report. In HAKI (Ed.), *Konstruksi Tahan Gempa Indonesia* (pp. 1–30). Jakarta: HAKI.
- BSN. (2013). Persyaratan Beton Struktural Untuk Bangunan Gedung-SNI 2847: 2013. Jakarta: Badan Standar Nasional.

Ersoy, U. (2013). A Simple Approach for Preliminary Design of Reinforced Concrete

Structures to be Built in Seismic Regions. *Teknik Dergi*, 24(4), 6559–6574.

- FEMA. (2007). *NEHRP Recommended Provisions for New Buildings and Other Structures: Training and Instructional Materials-FEMA 451B*. Washington DC: Federal Emergency Management Agency (FEMA).
- Mezzi, M. (2006). Enhancing the Seismic Performance of Existing "Pilotis" Configurations. In *IABSE Symposium Report* (Vol. 92, pp. 10–17).
- Okada, T., Murakami, M., Kabeyasawa, T., Katsumata, H., & Nakano, Y. (Eds.). (2005). *Guidelines for Seismic Retrofit of Existing Reinforced Concrete Buildings*. Tokyo: The Japan Building Disaster Prevention Association (JBDPA).
- Seki, M. (2015). A Proposal on the Simplified Structural Evaluation Method for Existing Reinforced Concrete Buildings Based on the Japanese Seismic Evaluation Standard vis-a-vis the International Seismic Code. *Journal of Earthquake Science and Engineering*, 2(1), 14–24.

http://www.perencanaanstruktur.com/2010/10/bentuk-keruntuhan-bangunan-saatgempa.html.